

# Mathematical methods for engineering. The finite element method

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## FEM: Remarks on programming

$$\begin{cases} -\Delta u = f & \text{on } \Omega = (0, 1) \times (0, 1) \\ u = 0 & \text{in } \partial\Omega \end{cases}$$

$$\begin{cases} \text{Find } u \in V \text{ such that} \\ \int_{\Omega} \nabla u \cdot \nabla v \, dx = \int_{\Omega} f v \, dx \quad \forall v \in V \end{cases}$$

We consider a **triangulation** of the domain  $\mathcal{T}_h = \{K_k\}_{k=1}^{nel}$  and the **finite dimensional space**  $V_h$  of piecewise linear functions on  $\mathcal{T}_h$ .

$$\begin{cases} \text{Find } u_h \in V_h \text{ such that} \\ \int_{\Omega} \nabla u_h \cdot \nabla v_h \, dx = \int_{\Omega} f v_h \, dx \quad \forall v_h \in V_h \end{cases}$$

# The linear system

Let  $\{\phi_i\}_{i=1}^{\text{ndof}}$  be a base of  $V_h$ . Then  $u_h(x) = \sum_{j=1}^{\text{ndof}} U_j \phi_j(x)$  and in particular

$$\int_{\Omega} \nabla \left( \sum_{j=1}^{\text{ndof}} U_j \phi_j(x) \right) \cdot \nabla \phi_i(x) dx = \int_{\Omega} f(x) \phi_i(x) dx \quad \forall i = 1, \dots, \text{ndof}$$

$$\sum_{j=1}^{\text{ndof}} U_j \int_{\Omega} \nabla \phi_j(x) \cdot \nabla \phi_i(x) dx = \int_{\Omega} f(x) \phi_i(x) dx \quad \forall i = 1, \dots, \text{ndof}$$

$$a_{i,j} = \int_{\Omega} \nabla \phi_j(x) \cdot \nabla \phi_i(x) dx, \quad f_i = \int_{\Omega} f(x) \phi_i(x) dx$$

$$\mathbf{A} \mathbf{U} = \mathbf{f}$$

## The element matrix $A^K$ and the element load $\mathbf{f}^K$

$$a_{i,j} = \int_{\Omega} \nabla \phi_j(x) \cdot \nabla \phi_i(x) dx = \sum_{l=1}^{nel} \int_{K_l} \nabla \phi_j(x) \cdot \nabla \phi_i(x) dx$$

Each element stiffness matrices is a  $3 \times 3$  matrix.

$$S^K = \begin{bmatrix} s_{1,1} & s_{1,2} & s_{1,3} \\ s_{2,1} & s_{2,2} & s_{2,3} \\ s_{3,1} & s_{3,2} & s_{3,3} \end{bmatrix}$$

In order to calculate  $s_{i,j} = \int_K \nabla \phi_i \cdot \nabla \phi_j$  (local numeration) it is easier to work on the reference element.

Analogously

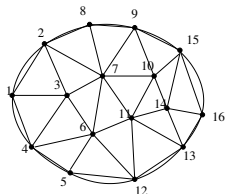
$$f_i = \int_{\Omega} f(x) \phi_i(x) dx = \sum_{l=1}^{nel} \int_{K_l} f(x) \phi_i(x) dx$$

and we can consider an element load  $\mathbf{f}^K$  that is a vector with three components.

# Main steps

- ▶ Construction and representation of the triangulation.
- ▶ Computation of the element stiffness matrices  $A^K$  and element loads  $\mathbf{f}^K$ .
- ▶ Assembly of the global stiffness matrix  $A$  and load vector  $\mathbf{f}$ .
- ▶ Solution of the system of equations  $A\mathbf{U} = \mathbf{f}$ .
- ▶ Postprocessing.

# Representation of the triangulation

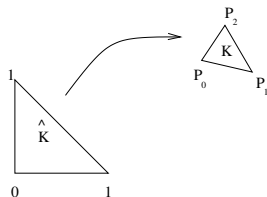


$$elem = \begin{bmatrix} 1 & 1 & 3 & 2 & \dots & 14 \\ 3 & 4 & 7 & 7 & \dots & 16 \\ 2 & 3 & 2 & 8 & \dots & 15 \end{bmatrix}$$

To represent a triangulation we will use:

- ▶ A matrix `coor` containing for each node its coordinates.
  - ▶ `coor` has two rows and `nnode` columns.
  - ▶ In this way we have numerated the nodes.
- ▶ Another matrix `elem` containing for each element its nodes.
  - ▶ `elem` has three rows and `nel` columns.
  - ▶ In this way we have a **local numeration** for the nodes of each triangle.

# The reference element



$$\hat{K} \rightsquigarrow K$$

This transformation makes

$$\begin{bmatrix} 0 \\ 0 \end{bmatrix} \rightsquigarrow \begin{bmatrix} p_0 \\ q_0 \end{bmatrix} \quad \begin{bmatrix} 1 \\ 0 \end{bmatrix} \rightsquigarrow \begin{bmatrix} p_1 \\ q_1 \end{bmatrix} \quad \begin{bmatrix} 0 \\ 1 \end{bmatrix} \rightsquigarrow \begin{bmatrix} p_2 \\ q_2 \end{bmatrix}$$

$$\mathbf{x} = B\hat{\mathbf{x}} + \mathbf{d} \quad \hat{\mathbf{x}} = B^{-1}(\mathbf{x} - \mathbf{d})$$

with

$$B = \begin{bmatrix} p_1 - p_0 & p_2 - p_0 \\ q_1 - q_0 & q_2 - q_0 \end{bmatrix} \quad \mathbf{d} = \begin{bmatrix} p_0 \\ q_0 \end{bmatrix}$$

# The element matrix

Concerning the gradients

$$\frac{\partial \phi_i}{\partial x_1} = \frac{\partial \hat{\phi}_i}{\partial \hat{x}_1} \frac{\partial \hat{x}_1}{\partial x_1} + \frac{\partial \hat{\phi}_i}{\partial \hat{x}_2} \frac{\partial \hat{x}_2}{\partial x_1} = \frac{\partial \hat{\phi}_i}{\partial \hat{x}_1} m_{1,1} + \frac{\partial \hat{\phi}_i}{\partial \hat{x}_2} m_{2,1}$$

where  $M = B^{-1}$ . Analogously

$$\frac{\partial \phi_i}{\partial x_2} = \frac{\partial \hat{\phi}_i}{\partial \hat{x}_1} m_{1,2} + \frac{\partial \hat{\phi}_i}{\partial \hat{x}_2} m_{2,2}$$

Hence

$$\int_K \nabla \phi_i \cdot \nabla \phi_j \, d\mathbf{x} = |\det(B)| \int_{\hat{K}} B^{-T} \nabla \hat{\phi}_i \cdot B^{-T} \nabla \hat{\phi}_j \, d\hat{\mathbf{x}}$$



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Hence

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Concerning the functions

$$\int_K \phi_i \phi_j \, d\mathbf{x} = |\det(B)| \int_{\hat{K}} \hat{\phi}_i \hat{\phi}_j \, d\hat{\mathbf{x}}$$

## selem

```
function s=selem(K)
for i=1:2
    B(:,i)=K(:,i+1)-K(:,1);
end
GFref=[-1 1 0; -1 0 1]; % each column is the gradient
                        % of the corresponding base function
                        % in the reference element.
GF=inv(B) '*GFref;
for i=1:3
    for j=1:3
        s(i,j)=GF(:,i) '*GF(:,j);
    end
end
s=s*abs(det(B))/2;      % 1/2 is the area of the reference
                        % element
```

# Assembly of the global stiffness matrix $A$

$$elem = \begin{bmatrix} 1 & 2 & 2 & \dots & 30 \\ 2 & 8 & 3 & \dots & 36 \\ 7 & 7 & 8 & \dots & 35 \end{bmatrix}$$

```
for l=1:number of elements
calculate the stiffness matrices of the element
  for i=1:3
    for j=1:3
      A(elem(i,l),elem(j,l))=A(elem(i,l),elem(j,l))+s(i,j)
    end
  end
end
```

## The load vector $\mathbf{f}$

- ▶ For each element there is a  $3 \times 1$  load vector

$$\mathbf{f}_i^K = \int_K f(\mathbf{x}) \phi_i(\mathbf{x}) d\mathbf{x} \quad i = 1, 2, 3.$$

- ▶ They are calculated using numerical integration.

Then the vector  $\mathbf{f}$  is assembly

```
for l=1:number of elements
calculate the load vector of the element
  for i=1:3
    f(elem(i,l))=f(elem(i,l))+fel(i)
  end
end
```

] Three points quadrature rule:

$$\int_K g(x) dx \approx \frac{|K|}{3} \sum_{k=1}^3 g(P_k)$$

For  $i = 1, 2, 3$

$$f_i = \int_K f(x) \phi_i(x) dx \approx \frac{|K|}{3} \sum_{k=1}^3 f(P_k) \phi_i(P_k) = \frac{|K|}{3} f(P_i)$$

because  $\phi_i(P_k) = \delta_{i,k}$ .

## felem - 2

```
function f=felem(K,fun)
for i=1:3
    x=K(1,i);
    y=K(2,i);
    f(i)=eval(fun);
end
for i=1:2
    B(:,i)=K(:,i+1)-K(:,1);
end
f=f*abs(det(B))/6;           % abs(det(B))/2 is the area of K
```

# Input data

- ▶ The matrix `coor` and the matrix `elem` giving the triangulation.
- ▶ A vector `bc` giving the nodes in the boundary.
- ▶ The load  $f$  (as a string).

# The program

```
function sol=femDir(coor,elem,bc,fun)
ndof=length(coor);
nelem=length(elem);
A=zeros(ndof);
F=zeros(ndof,1);
for l=1:nelem
    K=coor(:,elem(:,l));
    s=selem(K);
    f=felem(K,fun);
    for i=1:3
        for j=1:3
            A(elem(i,l),elem(j,l))=A(elem(i,l),elem(j,l))+s(i,j);
        end
        F(elem(i,l))= F(elem(i,l))+f(i);
    end
end
bound=find(bc(1,:));
intern=find(bc(1,:)-1);
A=A(intern,intern);
F=F(intern);
x=A\F;
sol=zeros(ndof,1);
sol(intern)=x;
sol(bound)=0;
```



## creadata

```
function [coor,elem,bc]=creadata(N)
ndof=(N+1)^2;
for j=1:N+1
    for i=1:N+1
        coor(1,(j-1)*(N+1)+i)=(i-1)/N;
        coor(2,(j-1)*(N+1)+i)=(j-1)/N;
    end
end
elem=[];
for j=1:N
    for i=1:N
        k=(j-1)*(N+1)+i;
        elemA=[k;k+1;k+N+1];
        elemB=[k+1;k+1+N+1;k+N+1];
        elem=[elem elemA elemB];
    end
end
```

## Example

Solve

$$\begin{cases} -\Delta u = 8\pi^2 \sin(2\pi x) \sin(2\pi y) & \text{in } \Omega = (0, 1) \times (0, 1) \\ u = 0 & \text{on } \partial\Omega \end{cases}$$

Solution:  $u = \sin(2\pi x) \sin(2\pi y)$ .